Second-order Direct Analysis Method to Code of Practice for the Structural Use of Steel 2011

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What?

Why?

When?

How?



Commonly used analysis and design method

- 6.6 FIRST-ORDER LINEAR ELASTIC ANALYSIS (FIRST-ORDER INDIRECT ANALYSIS)
- 6.6.1 General

 $P-\Delta$ and $P-\delta$ effects should be checked in the member design by the moment

- 6.7 SECOND-ORDER P-∆-ONLY ELASTIC ANALYSIS (SECOND-ORDER INDIRECT ANALYSIS)
- 6.7.1 General

This analysis method considers the changes in nodal coordinate and sway such that the

- 6.8 SECOND-ORDER P-∆-δ ELASTIC ANALYSIS (SECOND-ORDER DIRECT ANALYSIS)
- 6.8.1 General

Roth the P-Λ and P-δ effects are accounted for in the computation of hending moment in

When can I use first-order linear analysis method ??

Non-sway frames

Except for advanced analysis, a frame is classified as non-sway and the P-∆ effect can be ignored when

$$\lambda_{cr} \geq 10$$

Sway frames

Except for advanced analysis, a frame is classified as sway when

$$10 > \lambda_{cr} \geq 5$$

Sway ultra-sensitive frames

A frame is classified as sway ultra-sensitive when

$$\lambda_{cr} < 5$$

Only second order $P-\Delta-\delta$ or advanced analysis can be used for sway ultra-sensitive frames.

Elastic Critical Load factor

- 1) Deflection method
 - For sway buckling mode of a geometrically regular and rectangular frame

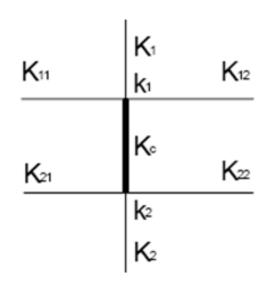
2) Eigenvalue analysis

Effective Length

Effective length of idealized columns

Table 8.6 - Effective length of idealized columns

Flexural Buckling						
Buckled shape of column shown by dashed line						1
Theoretical K value	0.5	0.7	1.0	1.0	2.0	1
Recommended K value when ideal conditions are approximated	0.70	0.85	1.20	1.00	2.10	1.5



Distribution factors:

$$k_1 = \frac{K_c + K_1}{K_c + K_1 + K_{11} + K_{12}}$$
$$k_2 = \frac{K_c + K_2}{K_c + K_2 + K_{21} + K_{22}}$$

Effective Length

How to define the buckling effective length

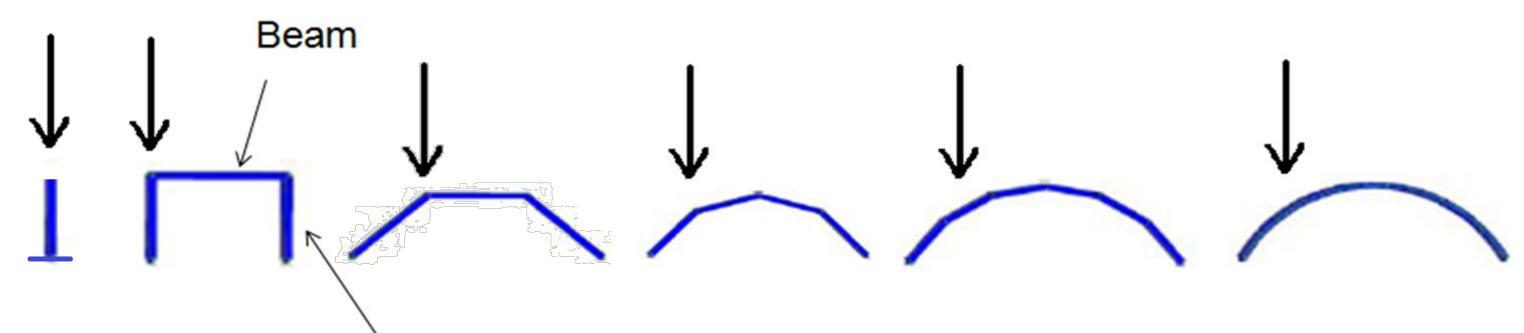
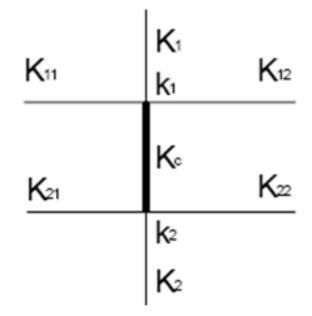


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$$\begin{array}{c|cccc} K_{11} & K_{1} & \\ & k_{1} & K_{12} \\ \hline & K_{2} & \\ & K_{22} & \\ & K_{2} & \\ & K_{2} & \\ & K_{2} & \\ \end{array}$$

Distribution factors:

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Moment Amplification for sway frames

$$\frac{\lambda_{cr}}{\lambda_{cr}-1}$$
:

Maximum slenderness ratio<200

What is Buckling?

Buckling behavior

Project: BENCHMARK EXAMPLE□

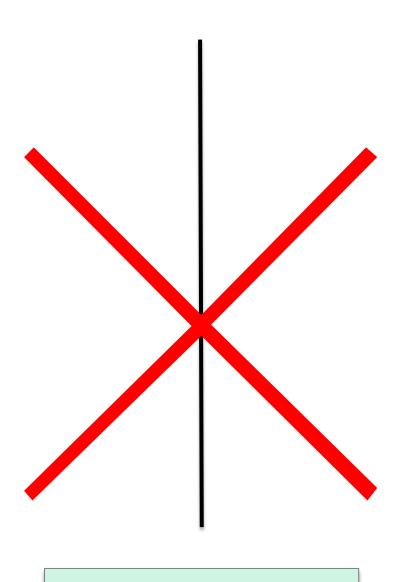
Unit: kN, m





Analysis Case(3), LoadStage(0), Load Cycle(10), Load Factor(57.23)

- > P-δ Moment
 - Member initial imperfection

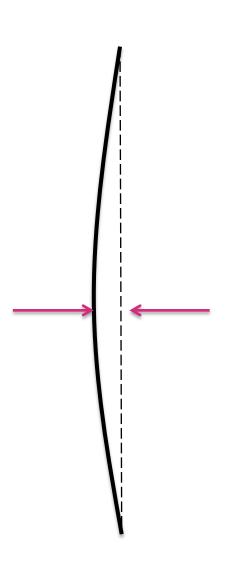


Perfect member

P-δ Moment

- Member initial imperfection

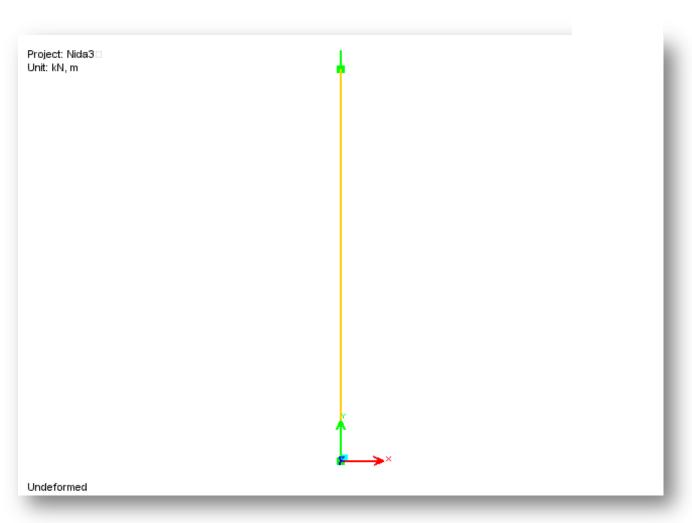
Buckling curves referenced in Table 8.7	$\frac{e_0}{L}$ to be used in Second-order P-Δ-δ elastic analysis
a ₀	1/550
a	1/500
b	1/400
С	1/300
d	1/200



Member Imperfection



- > P-δ Moment
 - Member initial imperfection + Member deformation under load

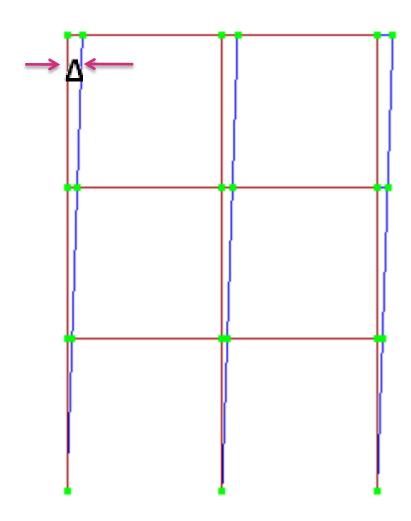


Use effective length and buckling reduction factor in linear analysis



P-Δ Moment

-Structural global imperfection

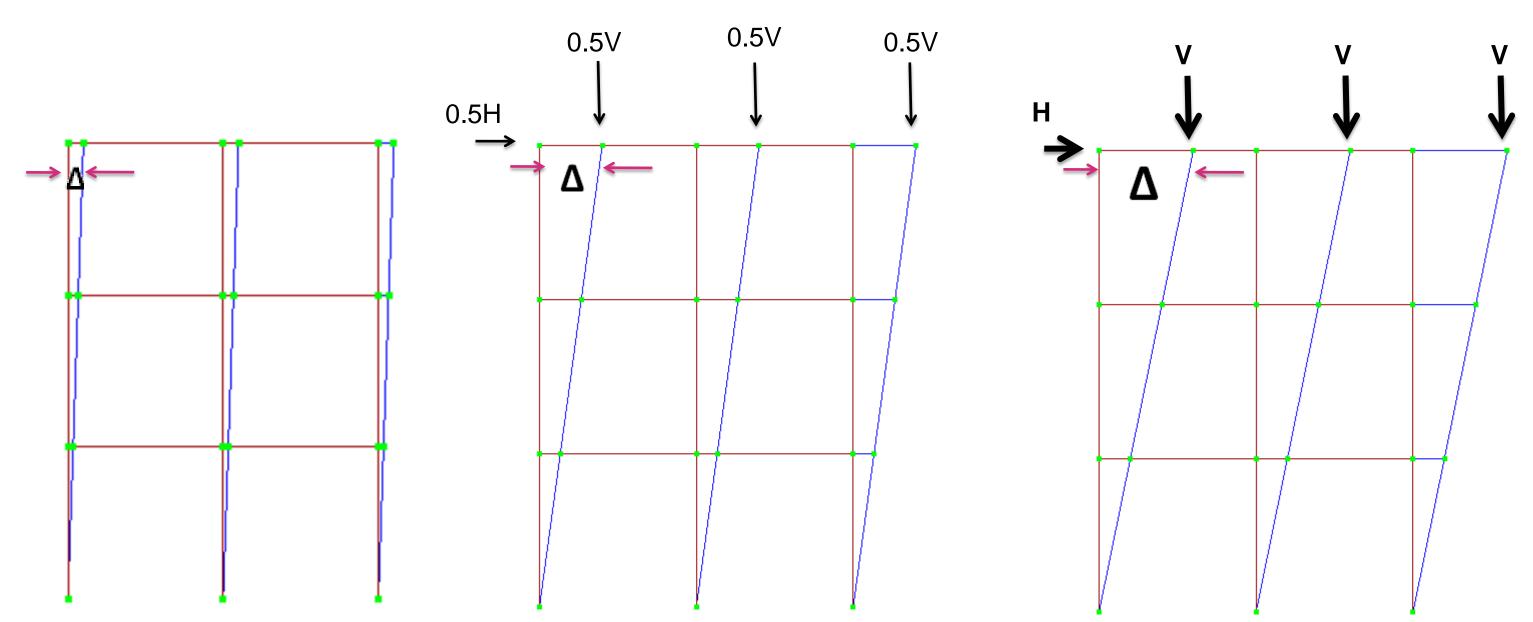


- Structural global imperfection
 - Shape of imperfection
 - 1) Notional horizontal force method
 - Suitable for regular structures
 - 2) Elastic buckling mode
 - Applicable for all structures

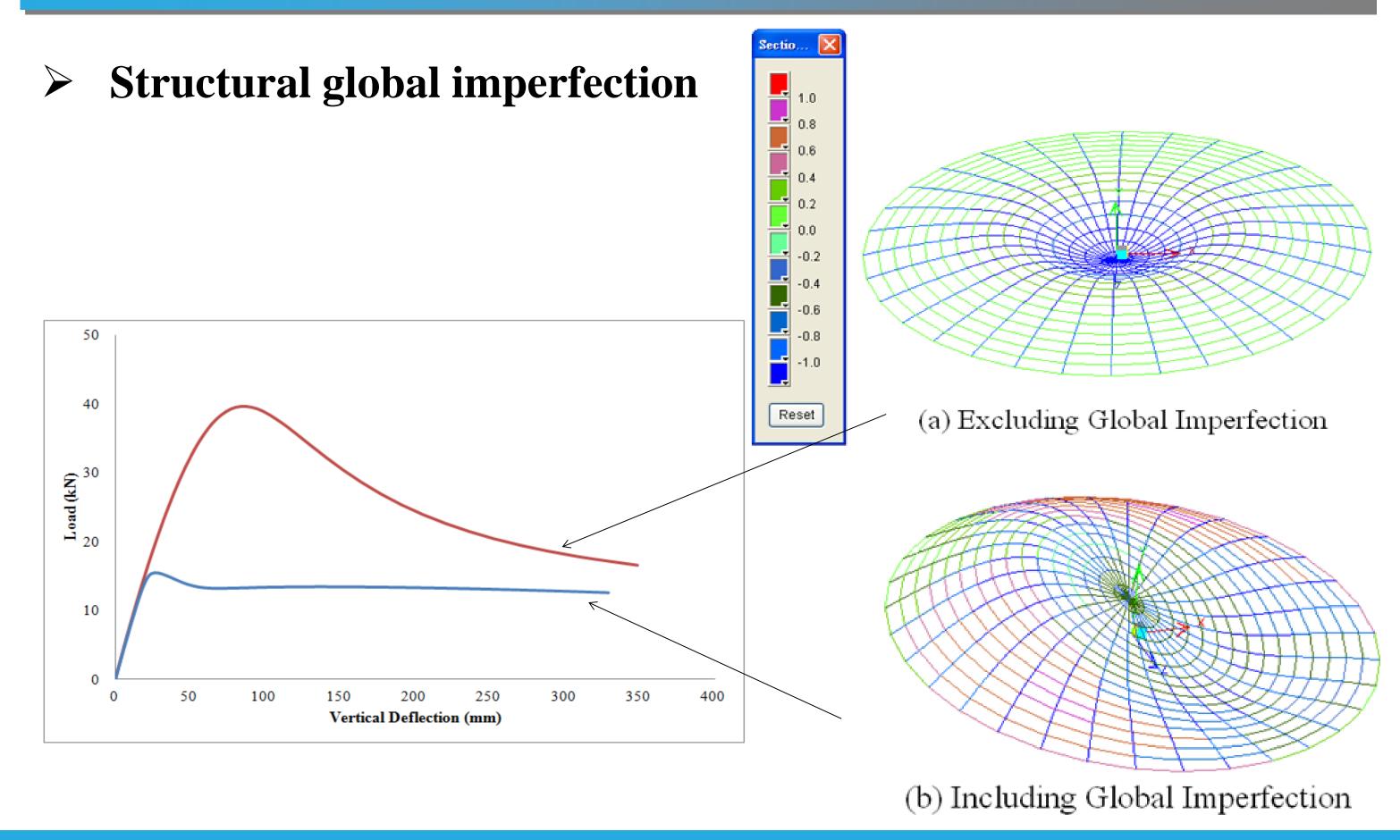
- Value of imperfection
 - Storey height/200

P-Δ Moment

-Structural global imperfection + Nodal geometric change



Use in Moment Amplification factor in linear analysis



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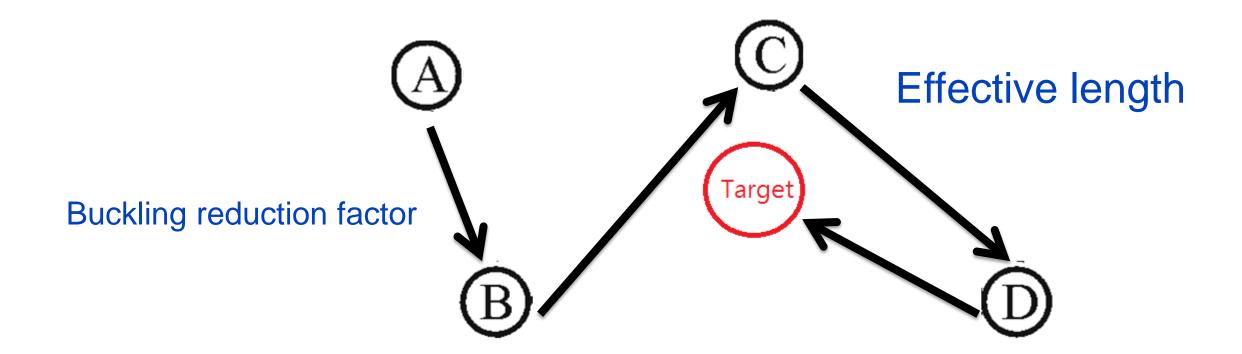
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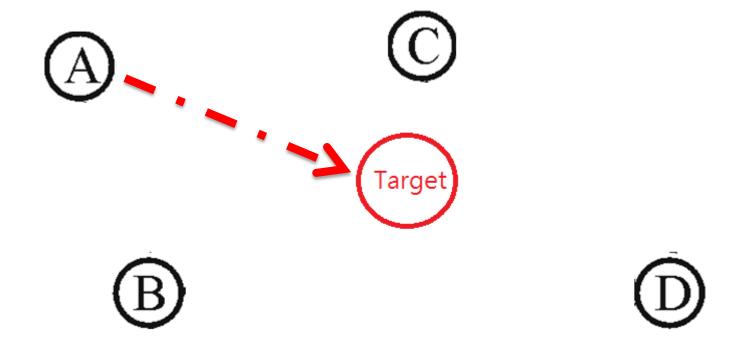
Roth the P-Λ and P-δ effects are accounted for in the computation of hending moment in



• What's second-order INDIRECT (P- Δ) analysis?



• What's second-order DIRECT (P- Δ - δ) analysis?



• What do I NEED for second-order direct analysis?

Member imperfection

Buckling curves referenced in Table 8.7	$\frac{e_0}{L}$ to be used in Second-order P-Δ-δ elastic analysis
a ₀	1/550
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Frame imperfect

Frame imperfections

The effects of imperfections for typical structures shall be incorporated in frame analysis using an equivalent geometric imperfection in Equation 6.7 as an alternative to the <u>notional horizontal force</u> in clause 2.5.8,

 $\Delta = h / 200$

Section capacity check

$$\frac{F_c}{A_g p_y} + \frac{M_x}{M_{cx}} + \frac{M_y}{M_{cy}} = \frac{F_c}{A_g p_y} + \frac{\overline{M}_x + F_c(\Delta_x + \delta_x)}{M_{cx}} + \frac{\overline{M}_y + F_c(\Delta_y + \delta_y)}{M_{cy}} \le 1$$

$$\left(\frac{M_x}{M_{rx}}\right)^{z_1} + \left(\frac{M_y}{M_{ry}}\right)^{z_2} = \left(\frac{\overline{M}_x + F_c(\Delta_x + S_x)}{M_{rx}}\right)^{z_1} + \left(\frac{\overline{M}_y + F_c(\Delta_y + S_y)}{M_{ry}}\right)^{z_2} \le 1$$

Comment questions

Second-order direct analysis ONLY used in HK Steel Code?

Comment questions

Recommended by many design codes



Comment questions

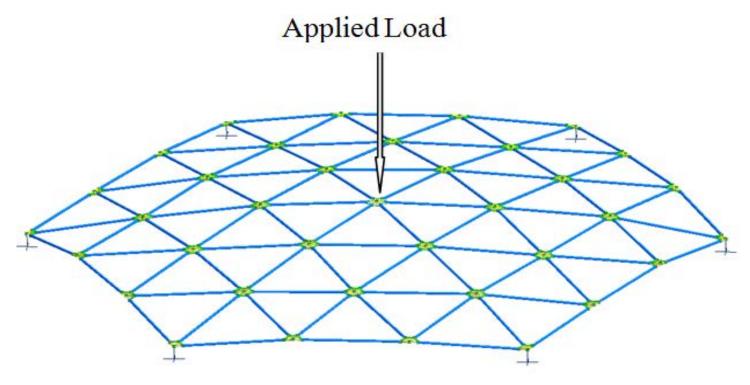
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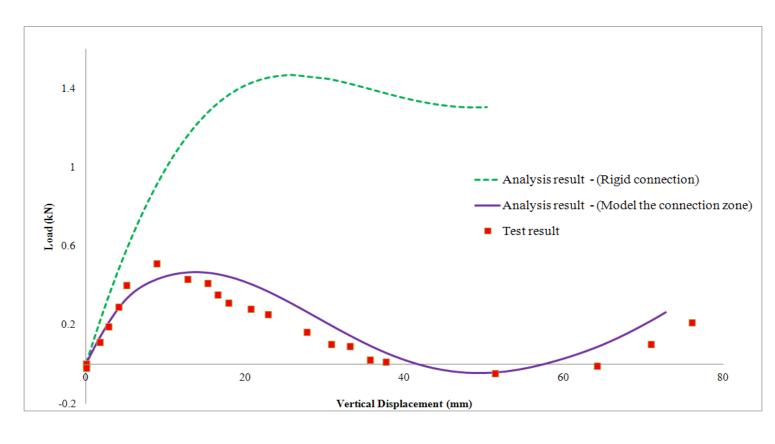
Second-order analysis gives a more economic/conservative design?

Can I add the reaction forces from each load cases?

Anything miss in linear analysis?

Snap-through buckling problem







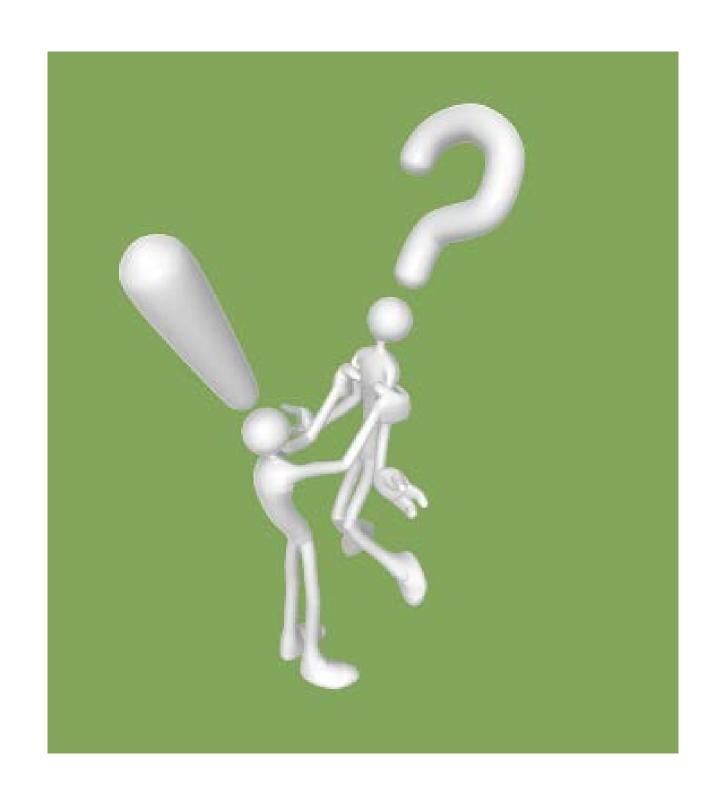
Conclusion

What?

Why?

When?

How?



Thank you

Young Members Group (YMG)

Hong Kong Institute of Steel Constriction (HKISC)